One Dalton Hotel & Residences: Implementation of ground movement control measure for a deep excavation in Boston blue clay

Mark X. Haley P.E., M.ASCE Jean Louis Z. Locsin Ph.D., P.E. Jesse D. Siegel P.E.

March 26, 2019



Outline

- 1 One Dalton Hotel & Residences
- 2 Challenges
- 3 Subsurface conditions
- 4 Excavation and Support of Excavation
- 5 Lateral and vertical movements
- 6 Conclusions

One Dalton Hotel & Residences

- Five-star hotel (Four Seasons) and luxury condos
- Back Bay Area of Boston, MA, USA
- Developer:

 One Dalton Owner
 LLC/Carpenter &
 Company
- Architect:

 Pei Cobb Freed &
 Partners, and Cambridge

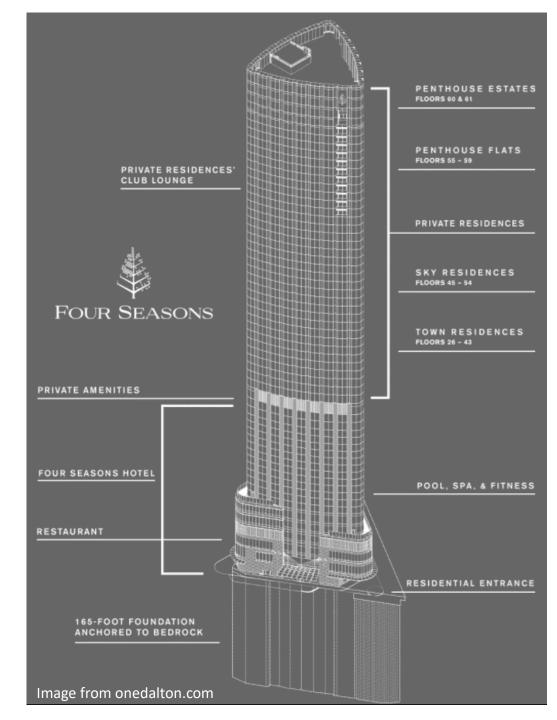
 Seven Associates
- Structural: WSP New York
- Geotechnical: Haley & Aldrich, Inc.



Rendered image from onedalton.com

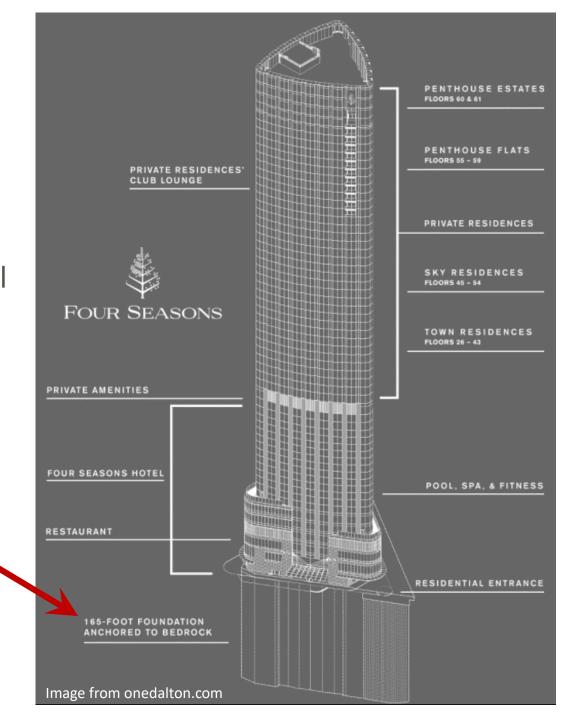
One Dalton Hotel & Residences

- Estimated completion:2019
- 61-story tower, 742 ft tall (3rd tallest in Boston when finished)
- 3 levels below grade

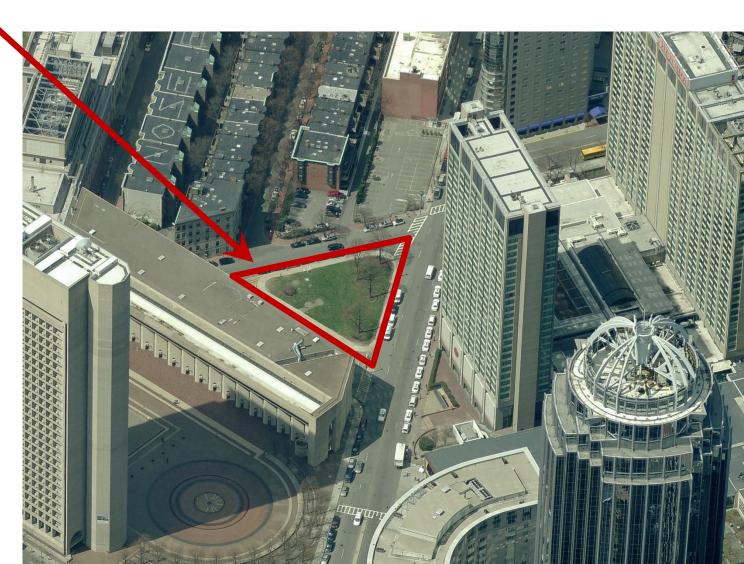


One Dalton Hotel & Residences

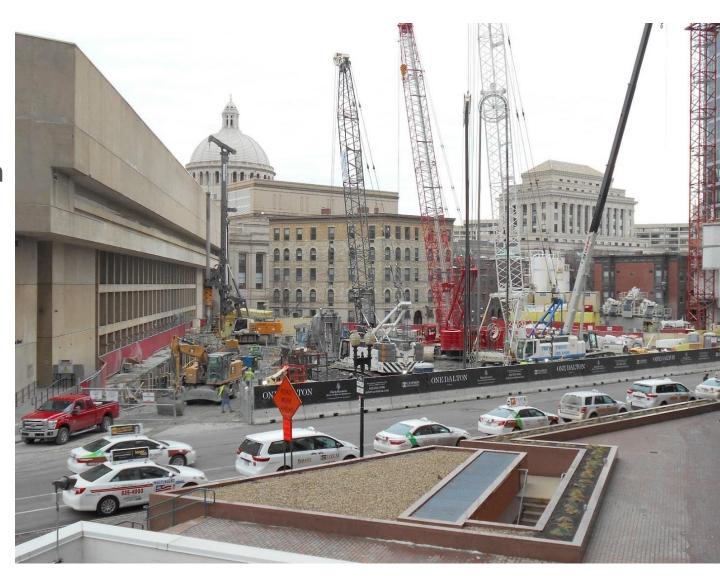
- Estimated Completion:2019
- 61-story tower, 742 ft tall (3rd tallest in Boston when finished)
- 3 levels below grade
- Rock-socketed deep foundations



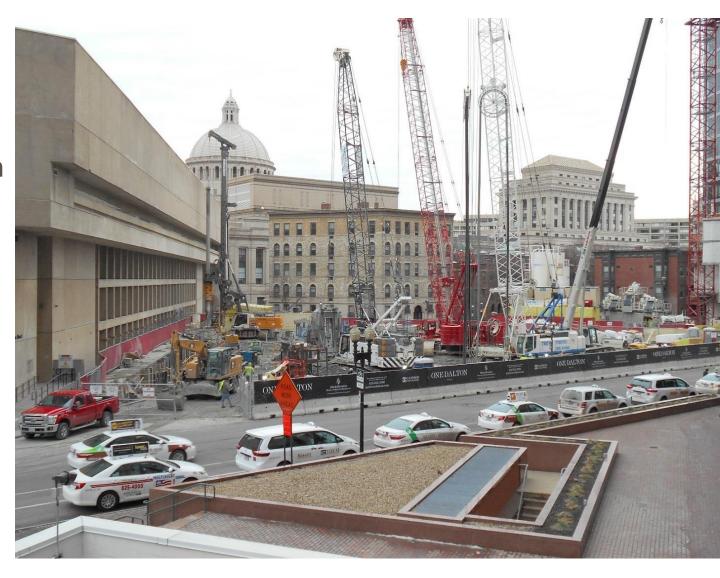
• Small site ~28,000 ft²



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- Urban construction



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- Deep excavation in soft soil

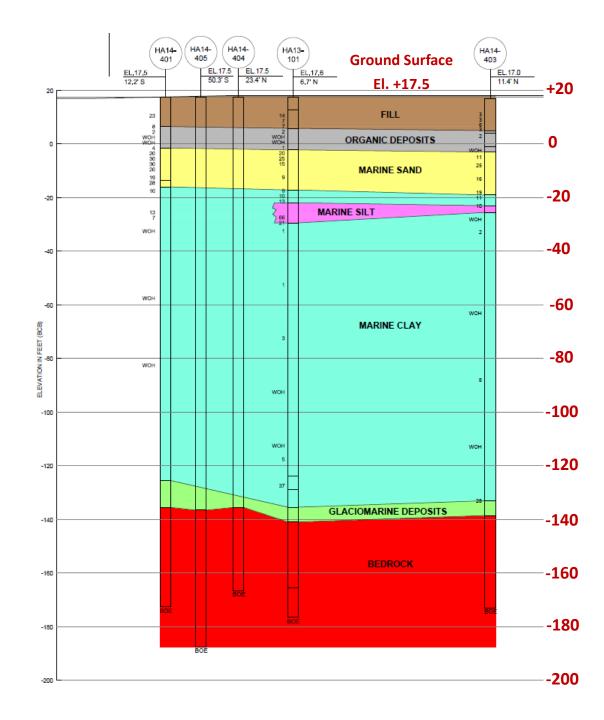


- Small site
 ~28,000 ft²
- Urban construction
- Deep excavation in soft soil
- Protection
 of adjacent
 structures
 on shallow
 foundations



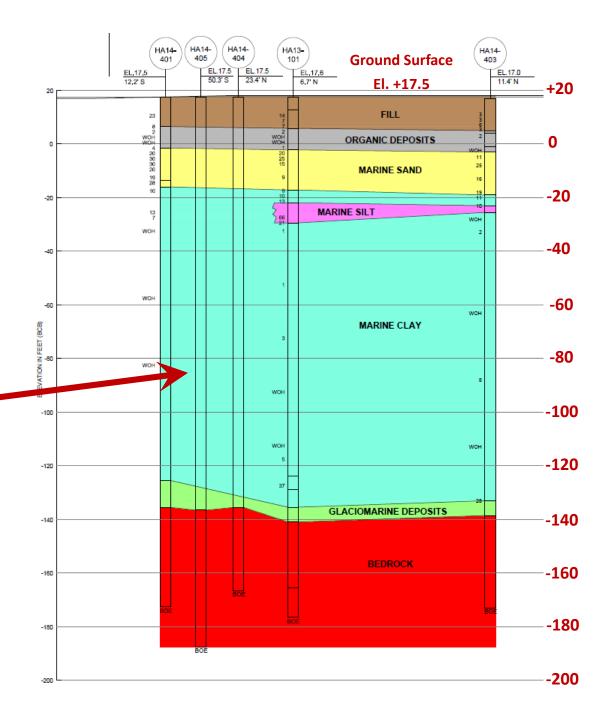
Subsurface conditions

- Fill
- Organic deposits
- Marine sand
- Marine silt
- Marine clay (Boston blue clay)
- Glaciomarine deposits
- Bedrock (argillite)



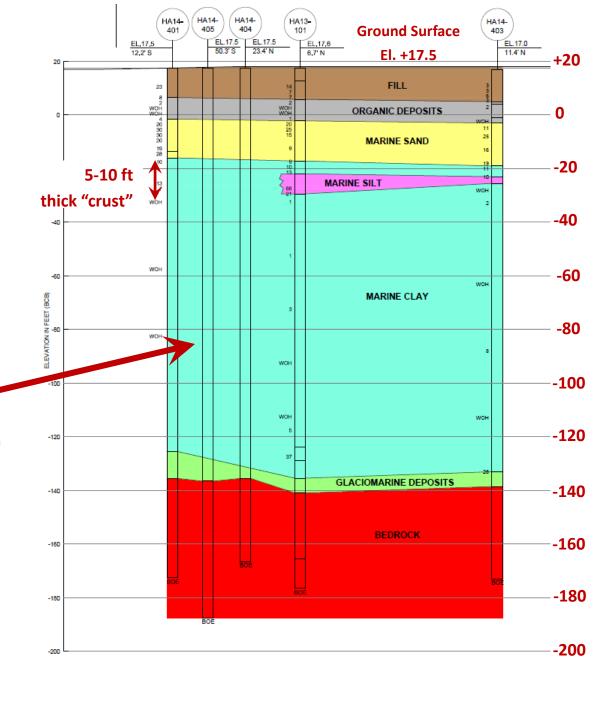
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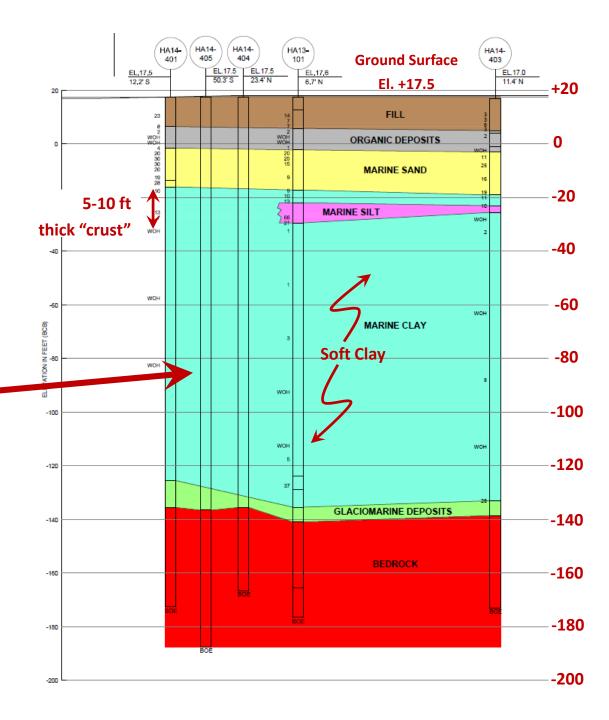
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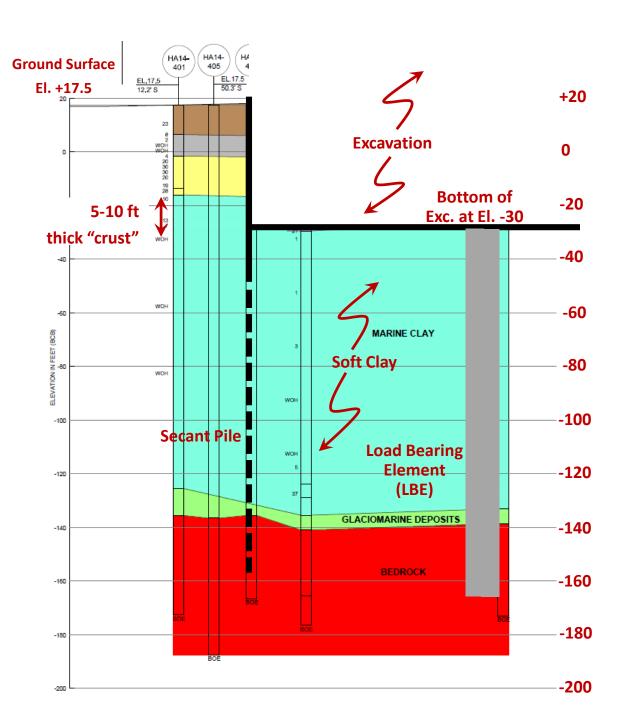
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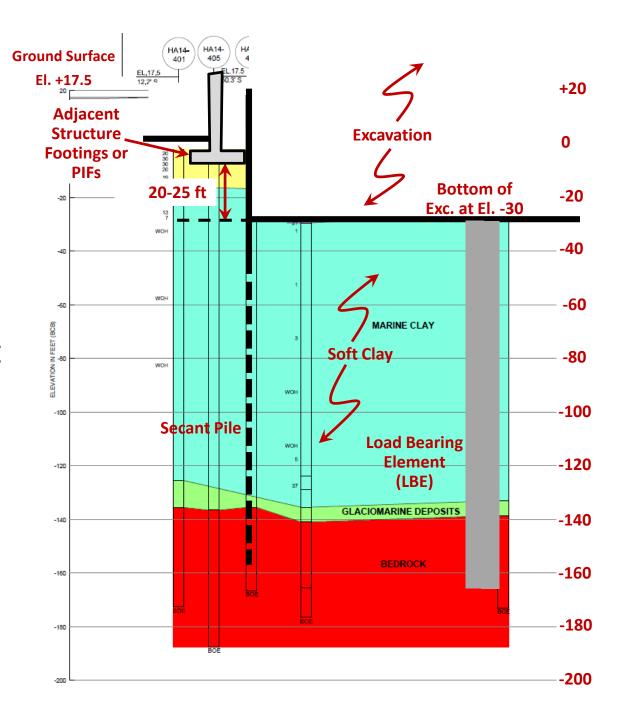
Excavation

- As deep as 48 ft below ground surface
- Bottom of excavation in soft clay

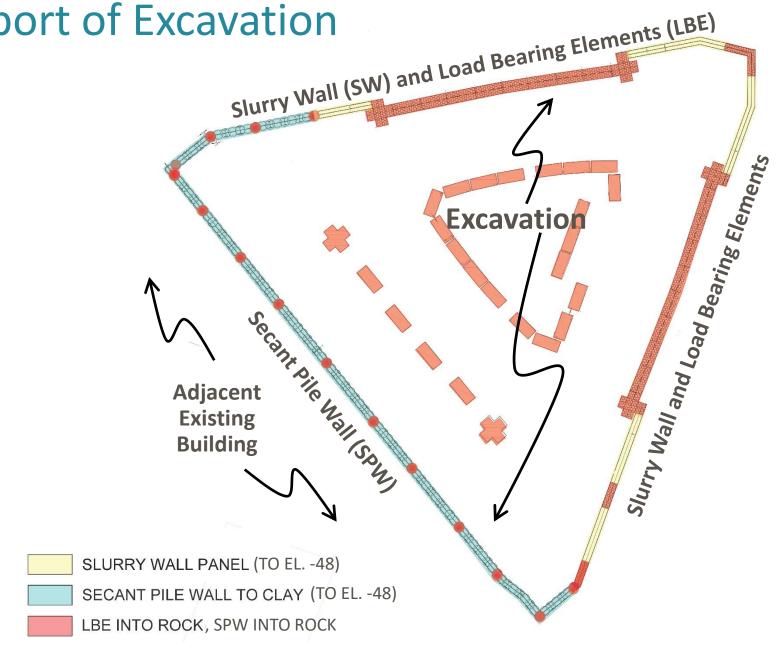


Excavation

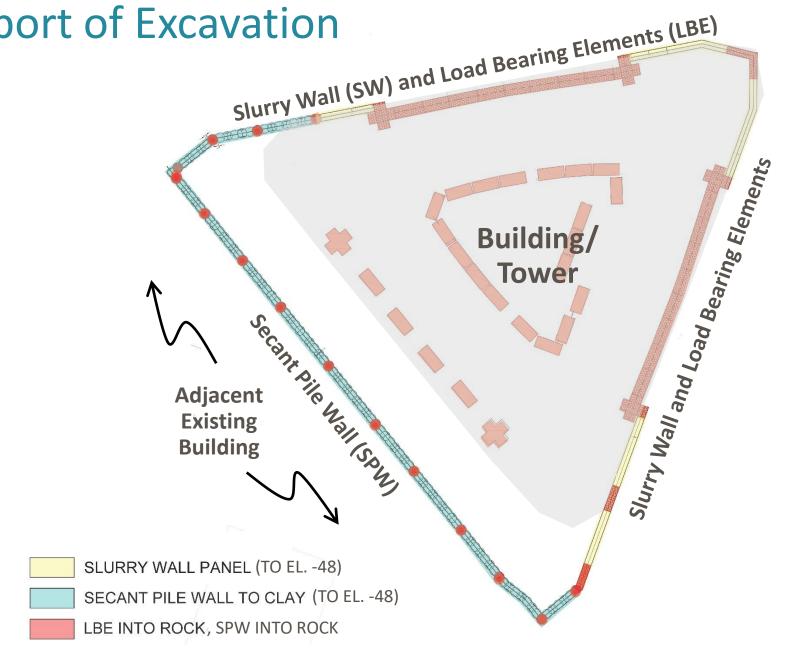
- As deep as 48 ft below ground surface
- Bottom of excavation in soft clay
- 20 to 25 ft below adjacent structure foundations



Support of Excavation

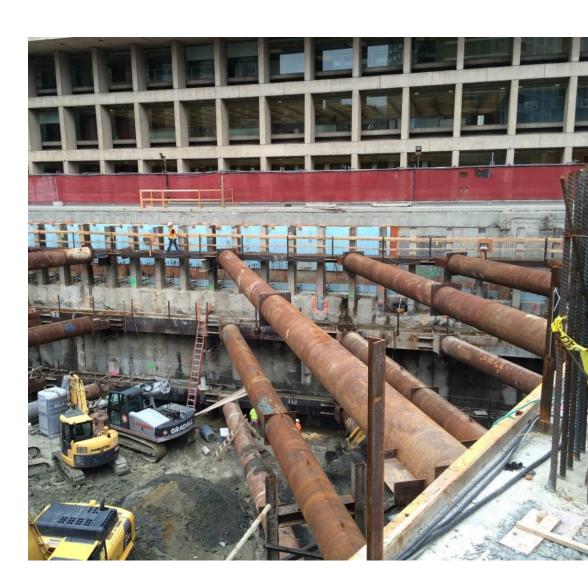


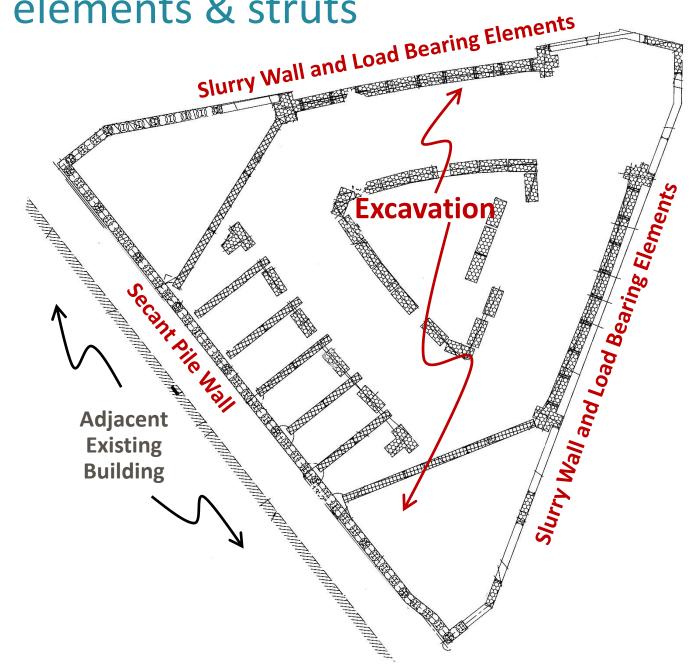
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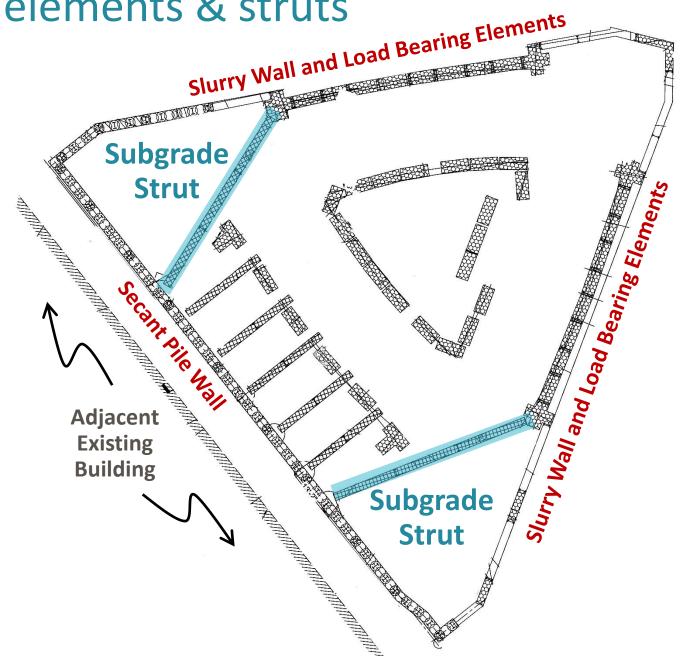
Support of Excavation

- The analysis for 3
 bracing levels
 predicted ~1.7 in.
 lateral deflection along
 SPW (adjacent to
 5-story building)
- Analysis indicated that to control movements, bottom lateral support would be required before excavation
- Solution: subgrade elements and struts

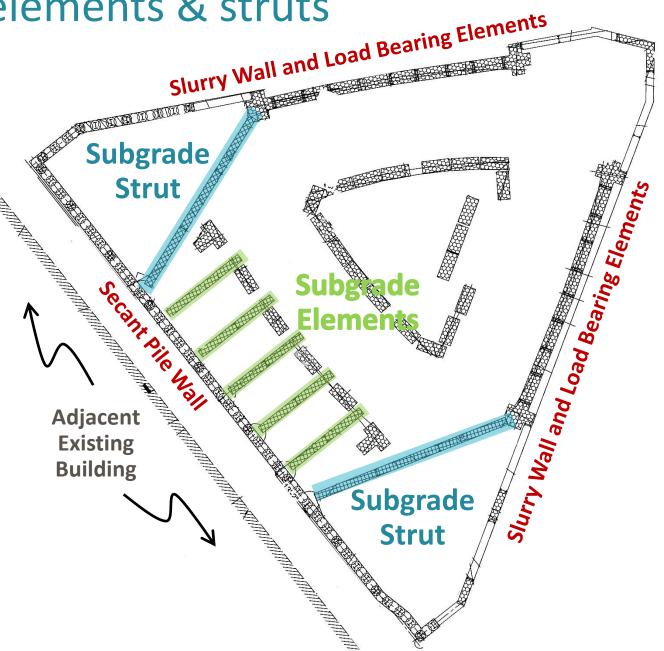




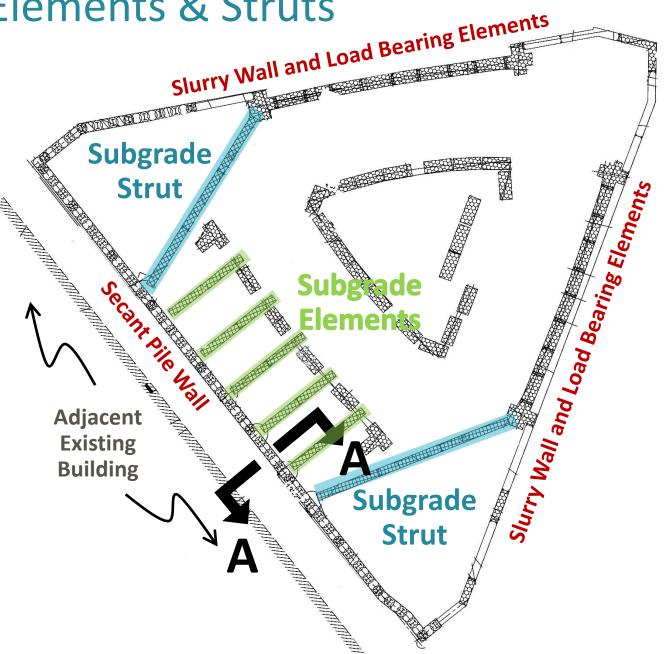
Two
 subgrade
 struts span
 from wall
 to LBE



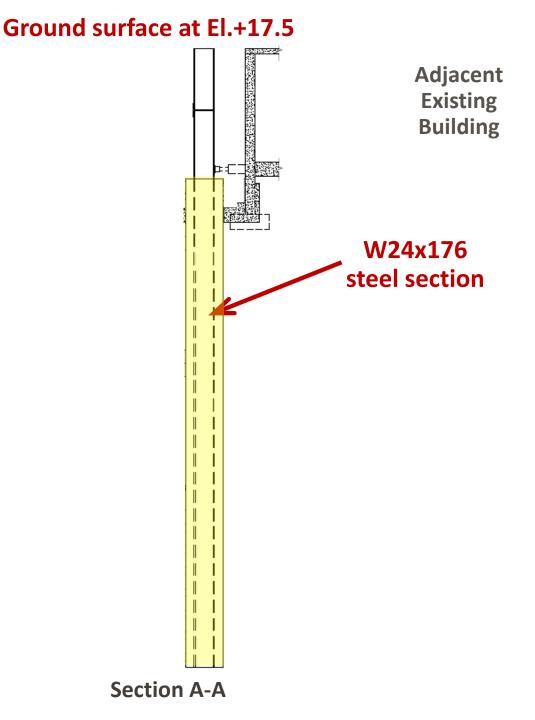
- Two
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- Five subgrade elements rely on skin friction (~30 ft)



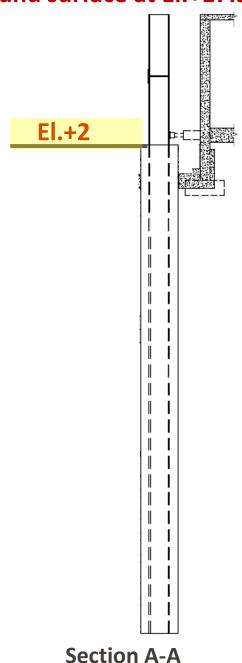
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 Secant pile wall (SPW) installed



Excavated to ~El.+2 **Ground surface at El.+17.5**

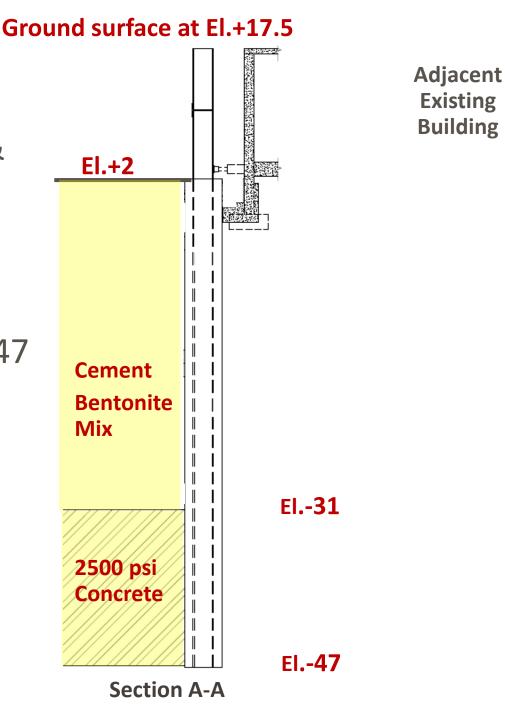


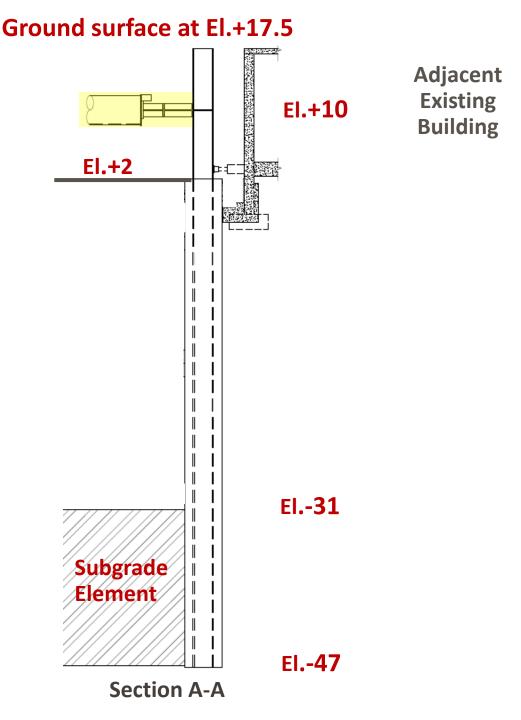
Adjacent Existing Building

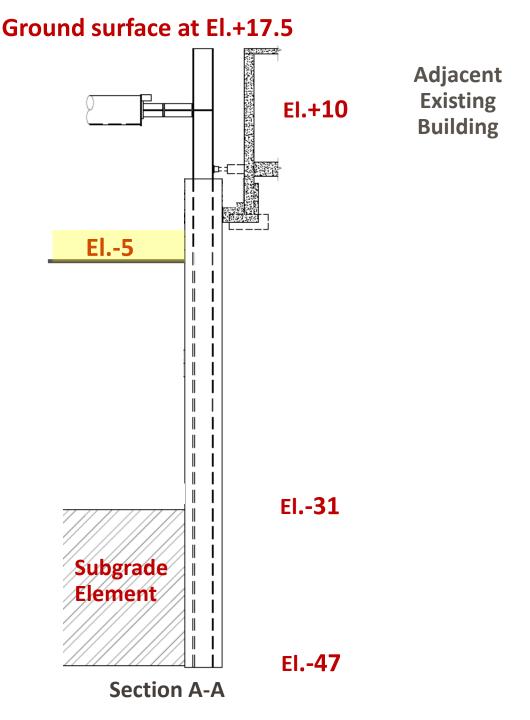
 From El.+2, excavate trenches for subgrade elements using clamshell bucket **Ground surface at El.+17.5 El.+2 Section A-A**

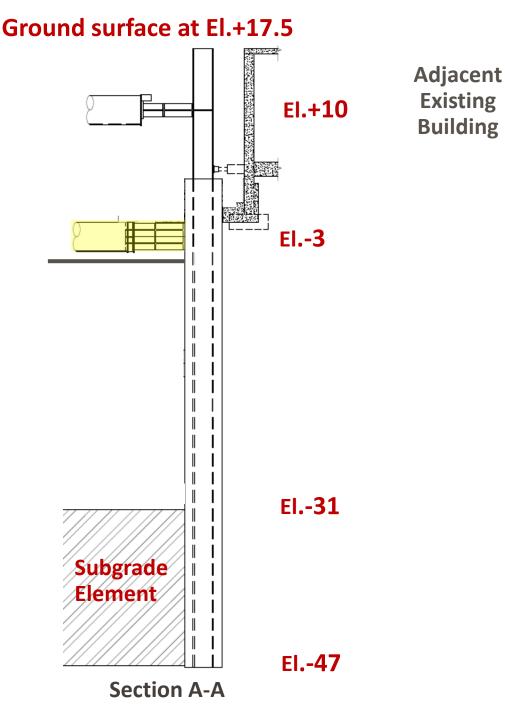
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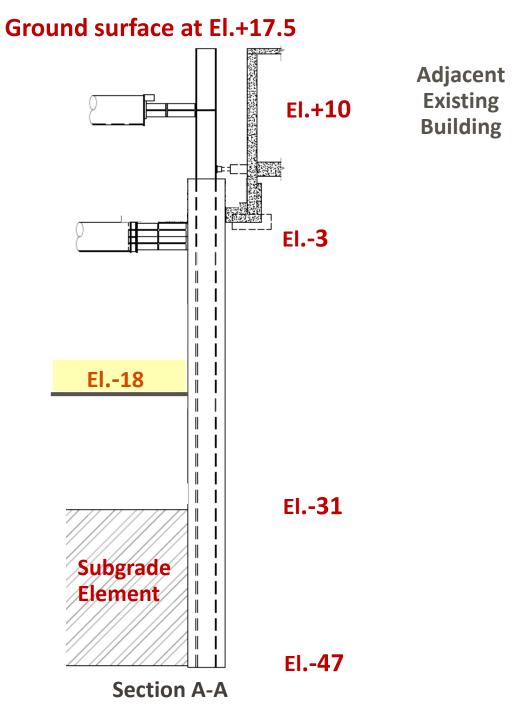
- Subgrade elements & struts installed from El.+2
- 2.5 ft wide, top at El.-31, bottom at El.-47 (16 ft high), 2500 psi concrete
- Cement bentonite above El. -31

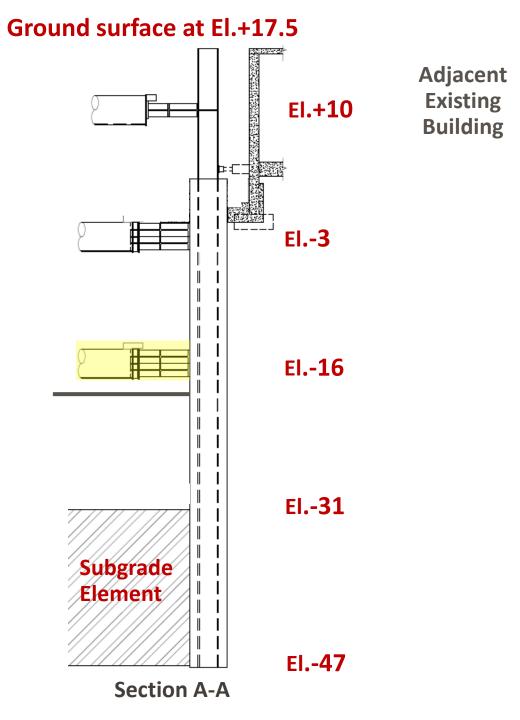


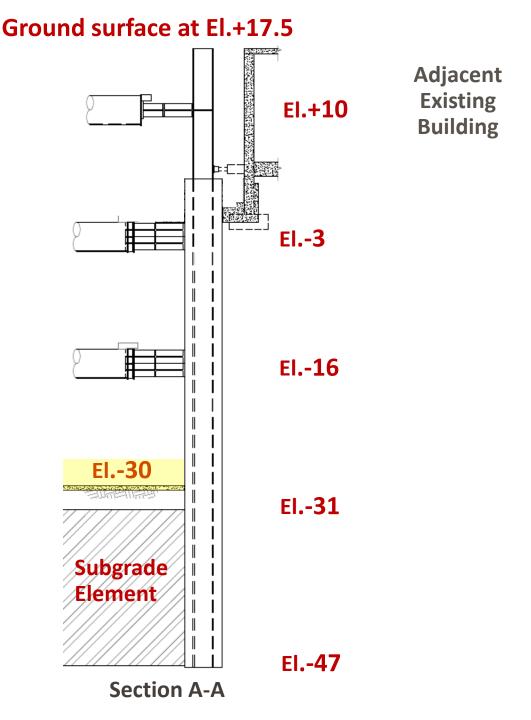






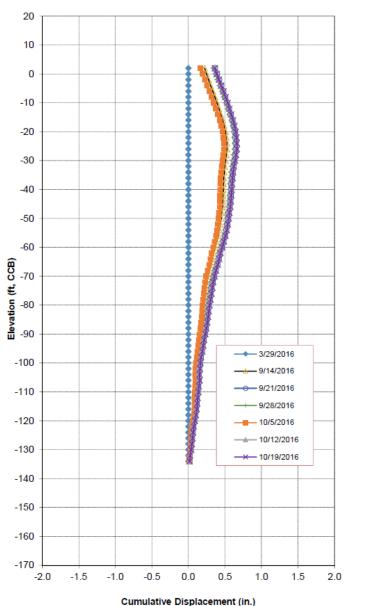




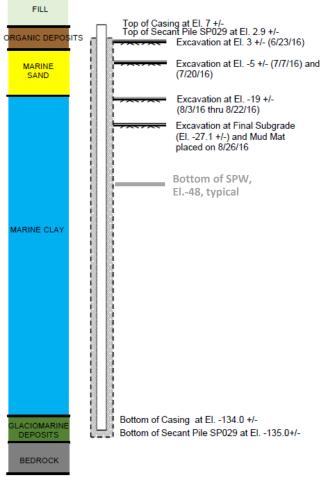


Wall lateral deflection – at end of excavation

 Deflection profile at Secant Pile Wall

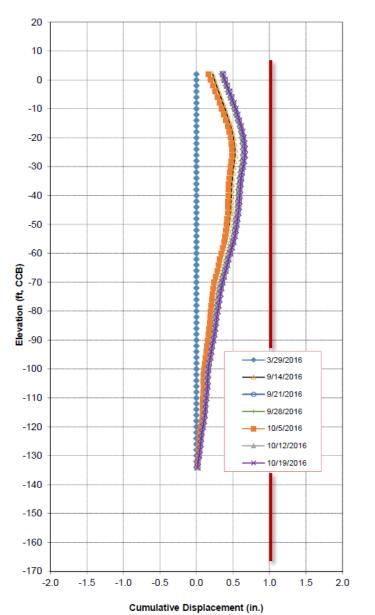


STATUS OF BELOW GRADE ACTIVITY



Wall lateral deflection – at end of excavation

- Deflection profile at Secant Pile Wall
- Less than1 inchlateraldeflection

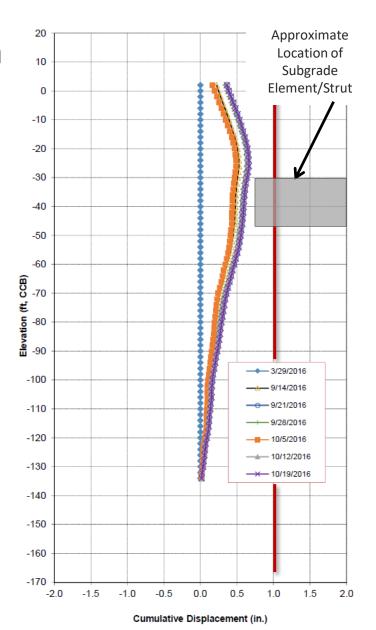


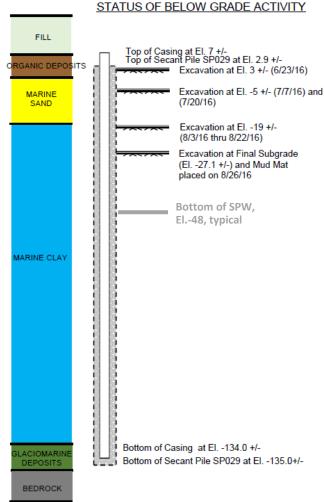
STATUS OF BELOW GRADE ACTIVITY FILL Top of Casing at El. 7 +/-Top of Secant Pile SP029 at El. 2.9 +/-GANIC DEPOSITS Excavation at El. 3 +/- (6/23/16) Excavation at El. -5 +/- (7/7/16) and MARINE SAND Excavation at El. -19 +/-(8/3/16 thru 8/22/16) Excavation at Final Subgrade (El. -27.1 +/-) and Mud Mat placed on 8/26/16 Bottom of SPW, El.-48, typical MARINE CLAY Bottom of Casing at El. -134.0 +/-LACIOMARIN Bottom of Secant Pile SP029 at El. -135.0+/-**DEPOSITS**

BEDROCK

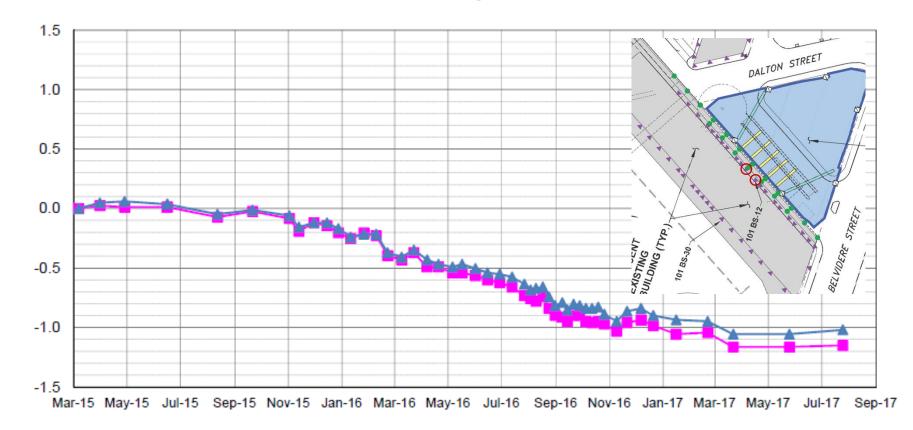
Wall lateral deflection – at end of excavation

- Deflection profile at Secant Pile Wall
- Less than1 inchlateraldeflection
- Effect of subgrade element/ strut is apparent

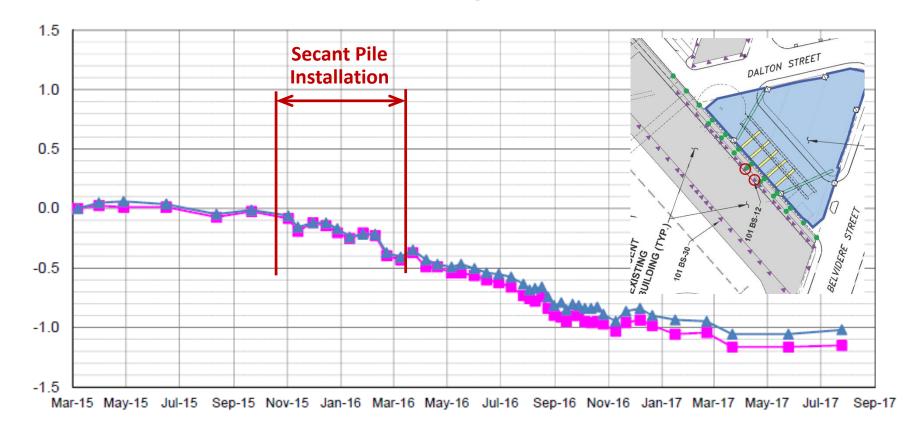




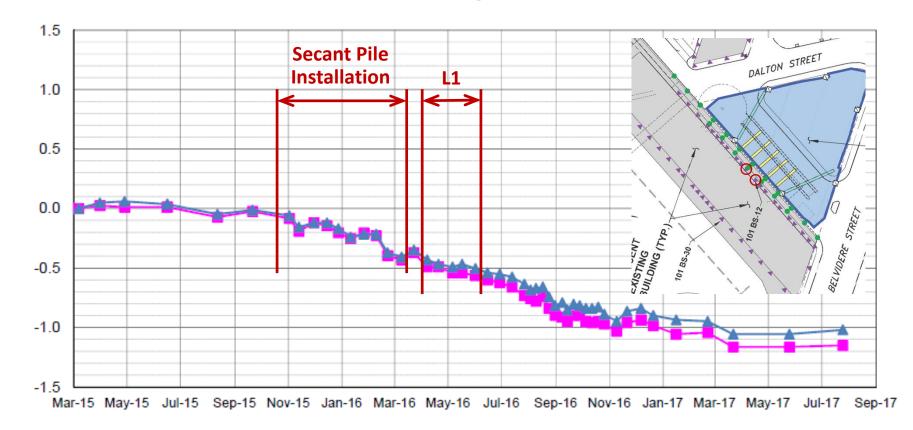
- Survey points on adjacent structure
- Movement monitored during construction



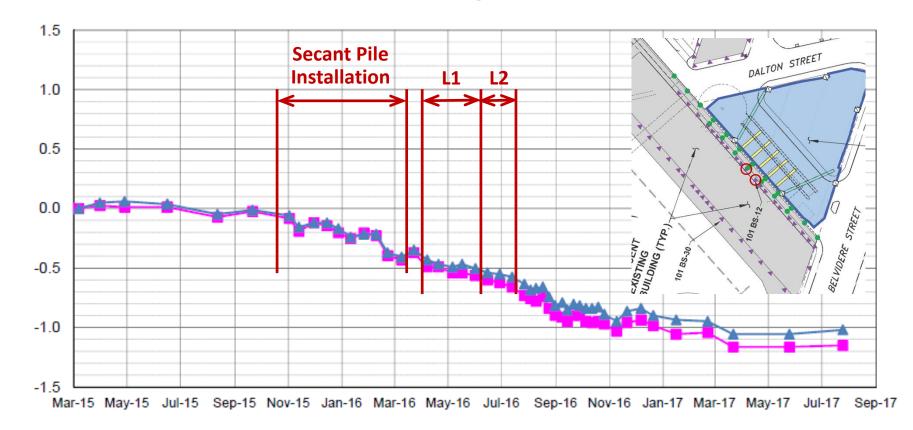
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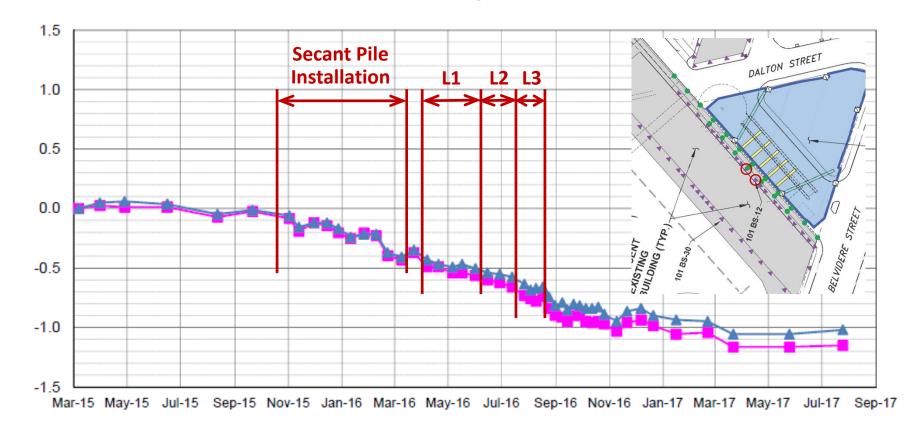
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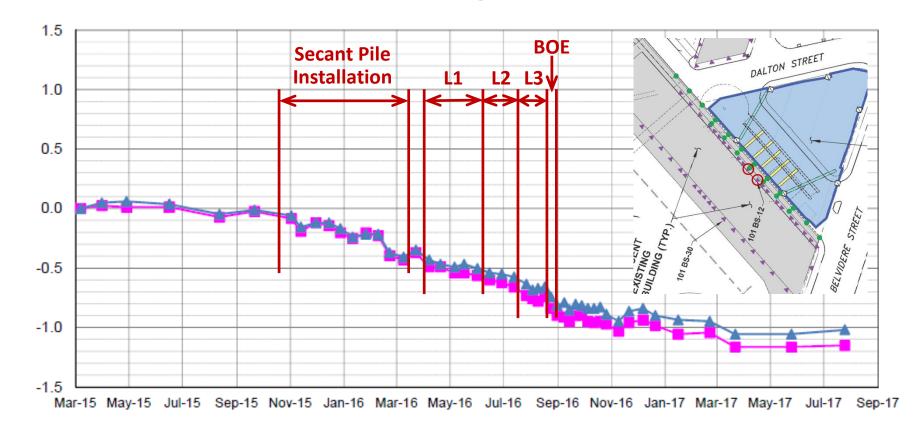
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- Survey points on adjacent structure
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Conclusions

- Excavating through clay crust increases risk of excessive movement
- To control movement, subgrade elements and struts installed before excavation
- Roughly 50% of measured movement occurred during installation of SPW (difficult to predict)
- Owner and Design Team chose to be proactive to mitigate movement and reduce risk by constructing subgrade elements and struts
- Subgrade elements and struts were effective in controlling lateral movement of wall below bottom of excavation and resulting vertical movement of adjacent structure



